

Design Optimization of Shear wall in High Rise Building

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Abstract - In past decades, shear walls are one of the most appropriate and important structural component in High Rise building. Therefore, it would be very interesting to study the structural response and their systems in High Rise Building structure. Shear walls contribute the stiffness and strength during earthquakes which are often neglected during design of structure and construction. This study shows the Optimization of shear walls in High Rise Building. In order to test, a G+40 storey building in seismic zone 2 was considered and analyzed for various parameters like base shear, storey drift, storey Displacement. Optimization of shear wall has been studied with the help of two models. For modeling and analysis of all the models, FEM based software ETABS 2018 were used. The analysis of all models was done by using Equivalent static method, Dynamics analysis and wind load Analysis. The comparison of results has been done based on same parameters like base shear, storey drift, lateral displacement & Base Shear.

Key Words: Shear wall, storey Drift, Lateral Displacement, Base shear.

1. INTRODUCTION

The rapid growth of the urban population and high cost of land creates a pressure on available space specially in developing cities. Tall buildings are the today's necessities today to fulfill these needs. As Designers' first choice is Shear wall as it resists the lateral loads such as wind or earthquake load and prevents buildings from damage. Shear wall is slender structure of having a large stiffness value, which resist lateral load and also resist the gravity load. Wind and seismic loads are the most common loads that shear walls are designed to carry. The distinguishing features are the much higher moment of inertia of the shear wall than a column and the width of the shear wall, which is not negligible in comparison with the span of adjacent beams Shear wall gives better performance when it is designed properly and placed at optimum location in the building plan

1.1 Objective

- To check the applicability of ductile design and detailing as per the code IS-13920-2016

- To check the applicability of minimum design lateral forces as per the code IS-1893-2016 at and IS-16700-2016
- Focus on the design optimization of vertical lateral load resisting member of shear wall.

1.2 Methodology

- Modeling & Analysis of a High rise Building With shear wall at different location for seismic loads and wind load.
- Comparison of Results and Graph of all models for storey displacement, storey drift, base shear, Storey shear.
- For Analysis Seismic zone 2 is considered.
- Optimization of shear wall in form of Reinforcement has calculated

2 MODELING AND ANALYSIS

Modelling a building involves the modelling and assemblage of its various load-carrying elements. The model must ideally represent the mass distribution, strength, stiffness and deformability. Modelling of the material properties and structural elements used in the present study is discussed below.

2.1 Building Description

Framed building located at Nagpur, Maharashtra, India (Seismic Zone II) is selected for the present study. The building is fairly symmetric in plan and in elevation.

Table -2.1 Building Description

Plan dimensions	33.35X20.25 m
Number of stories	G+40
Total height of building	147.6m
Height of each storey	3.6 m
Size of beams	230 X 500 mm (M40)

Thickness of shear wall	250 mm (M70/60/50/40)
Thickness of slab	200 mm (M40)
Partition walls	a) On floor 150 mm (AAC block, density 6kN/m ³) Wall Load- 2kN/m ² on floor
Seismic Data (IS1893 Part1-2016)	
Seismic zone	II
Soil condition	Hard soil
Importance Factor	1.2
Response Reduction Factor	3 and 5
Damping of structure	0.05
Wind load (IS 875 Part3-2015)	
Wind speed	44 m/s
Risk Coefficient (K1)	1
Terrain, height and structure size factor (K2)	As per condition
Topography factor(K3)	1
Importance factor for cyclonic region (K4)	1
Live load (IS875 Part2-1987)	2 kN/ m ²
Floor Finishing	2 kN/ m ²
Material	M 70/60/50/40 Grade concrete &Fe 650 Rebar
Unit weights	Concrete = 25 kN/m ³

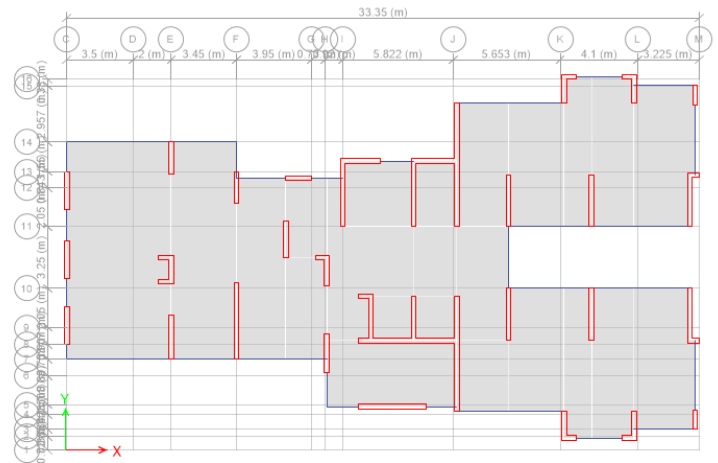


Fig 2.1 Typical floor plan of the selected building

2.2 Material Properties

Elastic material properties of these materials are taken as per Indian Standard IS 456: 2000.

A) Concrete:

Concrete with following properties is considered for study.

Characteristic compressive strength (f_{ck}) = 70/60/50/40 MPa

- ✓ Poisson Ratio = 0.3
- ✓ Density = 25 kN/m³
- ✓ Modulus of Elasticity (E) = $5000 \times \sqrt{f_{ck}}$
 M70 = 41833 MPa
 M60 = 38729 MPa
 M50 = 35355 MPa
 M40 = 31622 MPa

f_{ck} is the characteristic compressive strength of concrete cube in MPa at 28-day

B) Steel:

Steel with following properties is considered for study.

- ✓ Yield Stress (f_y) = 650 MPa
- ✓ Modulus of Elasticity (E) = 2×10^5 MPa

2.3 Analysis of Structure

A RCC medium rise building of G+40 stories with floor height 3.6m subjected to earthquake loading in Zone II has been considered. In this regard, ETABS software has been considered as tool to perform. Effect of shear wall on behavior of structural frames is analyzed. Storey displacements, story shear, story drift and base shear have been calculated to find out the effect in the building. Loading consideration Dead Load (DL) and Live load (LL) have been taken. Seismic load calculation has been done based on the IS 1893 (Part 1) (2016) approach. The loads combinations are applied to the selected frames are.

Load Combinations

Load Combination	LOAD FACTORS					
	D.L	L.L	WL X	WL Y	EQX	EQY
DL + LL	1.5	1.5	-	-	-	-
DL + LL + EQX	1.2	1.2	-	-	1.2	-
DL + LL - EQX	1.2	1.2	-	-	-1.2	-
DL + LL + EQY	1.2	1.2	-	-	-	1.2
DL + LL - EQY	1.2	1.2	-	-	-	-1.2
DL + EQX	1.5	-	-	-	1.5	-
DL - EQX	1.5	-	-	-	-1.5	-
DL +	1.5	-	-	-	-	1.5

EQY						
DL - EQY	1.5	-	-	-	-	-1.5
DL + LL + WLX	1.2	1.2	1.2	-	-	-
DL + LL - WLX	1.2	1.2	-1.2	-	-	-
DL + LL + WLY	1.2	1.2	-	1.2	-	-
DL + LL - WLY	1.2	1.2	-	-1.2	-	-
DL + WLX	1.5	-	1.5		-	-
DL - WLX	1.5	-	-1.5		-	-
DL + WLY	1.5	-	-	1.5	-	-
DL - WLY	1.5	-	-	-1.5	-	-
0.9DL + EQX	0.9	-	-	-	1.5	-
0.9DL - EQX	0.9	-	-	-	-1.5	-
0.9DL + EQY	0.9	-	-	-	-	1.5
0.9DL - EQY	0.9	-	-	-	-	-1.5
0.9DL + WLX	0.9	-	1.5		-	-
0.9DL - WLX	0.9	-	-1.5		-	-
0.9DL + WLY	0.9	-	-	1.5	-	-
0.9DL - WLY	0.9	-	-	-1.5	-	-

3. RESULTS & DISCUSSION

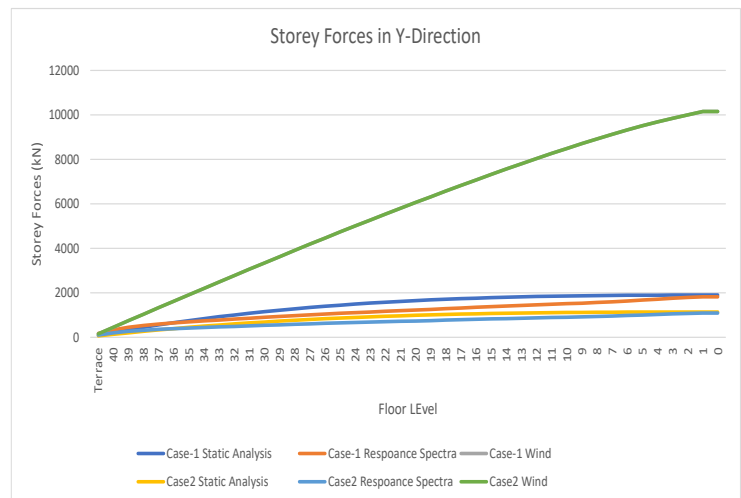
Table indicate the values of storey displacement, storey shear, base shear and storey drift for along X direction and Y direction as shown in Fig

Table 3.7 Seismic base Shear (KN)

Seismic Base shear					
LOAD CASE	STATIC BASE SHEAR(kN)		DYNAMIC BASE SHEAR (kN)		Min Vb (Cl.7.2.2) 0.7xW
	R=3	R=5	R=3	R=5	
X-Direction	2206	1323	2206	1323	1820
Y-Direction	1720	1032	1719.6	1032	1820

Table 3.8 Wind base Shear (KN)

Wind Base Shear	
LOAD CASE	STATIC BASE SHEAR(kN)
X- Direction	5410.0
Y- Direction	9439.0



OBSERVATIONS:-

- For easy comparison of the Storey displacement of the selected frames, plots of the storey displacement in transverse direction versus floor level in longitudinal direction are made, all imposed on the same graph. These are present in fig. 6.1 to fig. 6.2.
- Wind load case govern maximum displacement values in X and Y direction.
- In case1(R=3) and case2 (R=5) the displacement values are nearly same.
- Maximum Permissible limit is H/500 is 295.2 mm for wind and H/250 which is 590.4 mm for seismic.
- The displacement is inversely proportional to the stiffness.

3.1 Comparison of Results

RCC G+40 building is analyzed. Parameters like Storey displacement, storey shear, storey drift and base shear is calculated. Graphical representation of data is discussed in this chapter.

3.2 Graphical Representation For Storey Displacement

- The maximum displacement in longitudinal and transverse direction is considered and graphical representation of data is shown in Fig

Fig 3.1 Storey displacement along X-direction

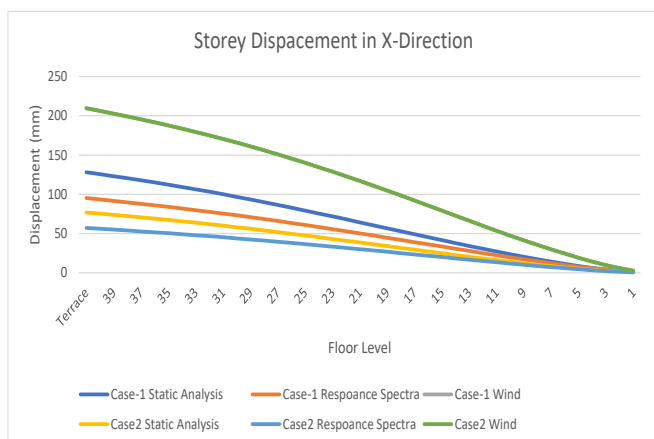


Fig 3.2 Storey displacement along Y-direction

3.3 Graphical Representation For Storey Shear

- The maximum storey shear in longitudinal and transverse direction is considered and graphical representation of data is shown in Fig

Fig 3.3 Storey forces along X-direction

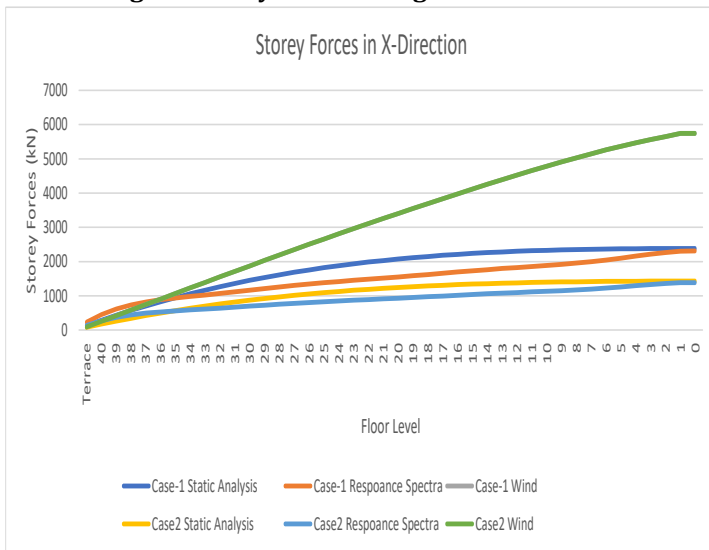
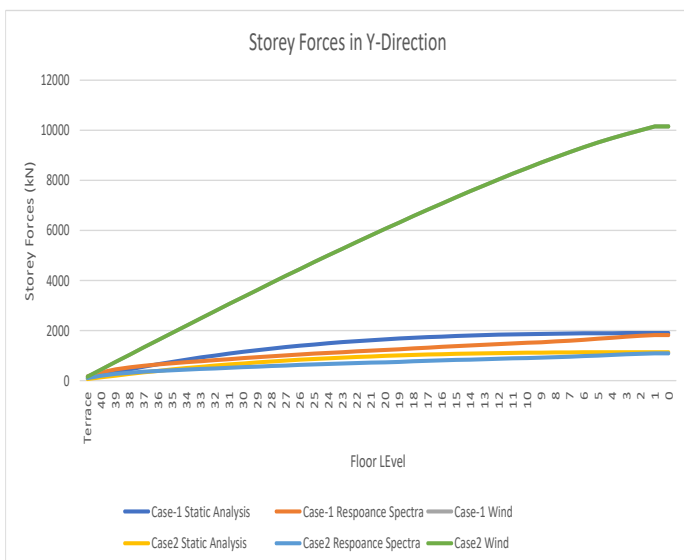


Fig 3.4 Storey forces along Y-direction



Observations:-

For easy comparison of the Storey shear of the selected frames, plots of the storey forces in transverse direction versus floor level in longitudinal direction are made, all imposed on the same graph. These are present in fig

- Wind load case govern maximum storey shear values in X and Y direction.
- In case1(R=3) and case2 (R=5) the storey shear values are nearly same

3.3 Graphical Representation for Storey Drift

- The maximum Storey Drift in longitudinal and transverse direction is considered and graphical representation of data is shown in Fig

Fig 3.5 Storey drift along X-direction

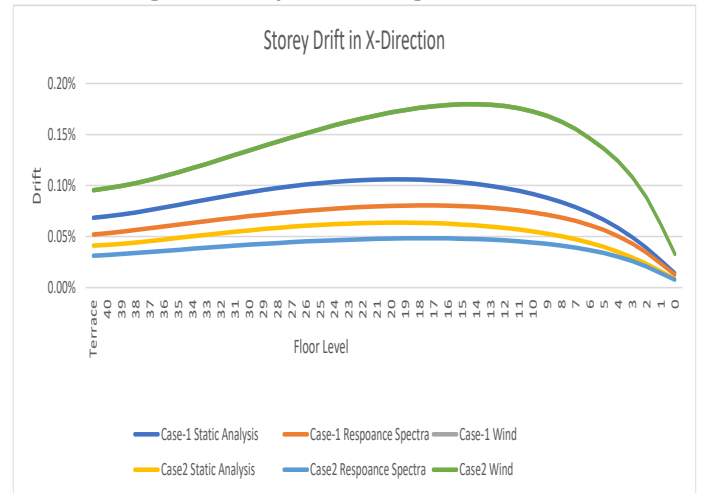
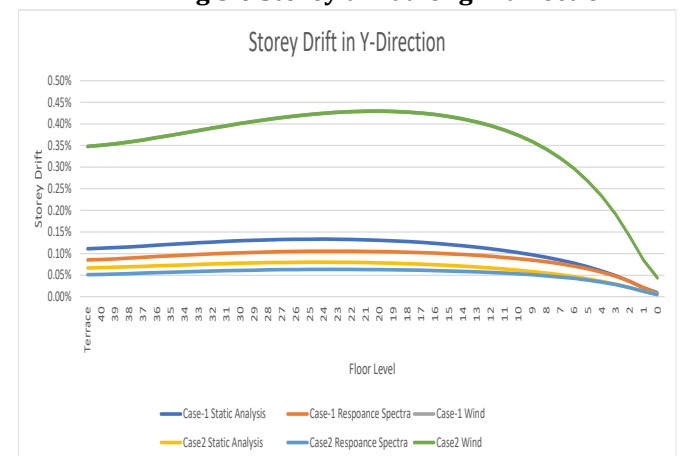


Fig 3.6 Storey drift along Y-direction



Observations:-

- For easy comparison of the Storey drift of the selected frames, plots of the storey drift in transverse direction versus floor level in longitudinal direction are made, all imposed on the same graph.
- Wind load case govern maximum storey drift values in X and Y direction.
- In case1 (R=3) and case2 (R=5) the storey drift values are nearly same.
- Maximum Permissible limit for storey drift for seismic behaviour is 0.004 times (i.e.0.4%) the

storey height but due to wind force storey drift governs the maximum value.

3.4 Graphical Representation for Base Shear

The Base Shear in longitudinal and transverse direction is considered and graphical representation of data is shown in

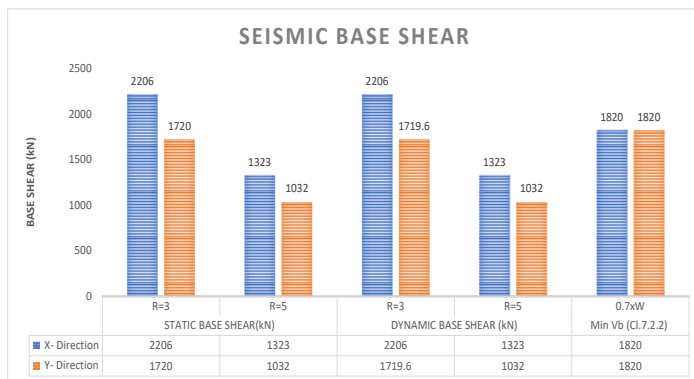


Fig 3.7 Seismic base shear

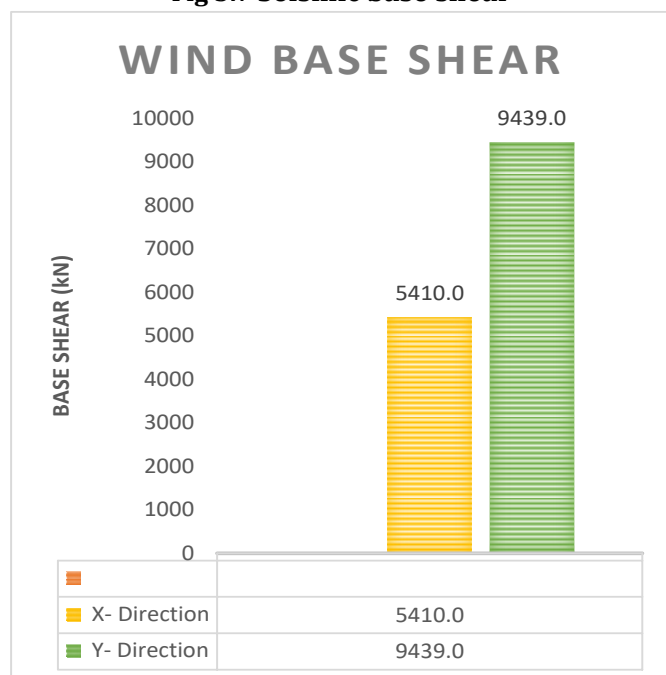


Fig 3.8 Wind base shear

Observations:-

- Plots of the base shear in longitudinal and transverse versus are made, all imposed on the same graph. The base shear is directly proportional to weight of structure.
- From the above graphs base shear profiles it is observed that minimum shear occurs in case 2.

- Wind load case govern maximum base shear values in X and Y direction.

4. CONCLUSIONS

- The result of storey displacement, storey shear, storey drift and base shear of high rise building is nearly same for case 1(R=3) and case2(R=5).
- Wind load case as compared to seismic load governs maximum storey displacement, storey shear, storey drift and base shear of high rise building and hence the design is done by taking wind effect only so that the ductile detailing is avoided.
- Building with shear wall is found to be very effective in increasing building performance.
- Wind base shear of the shear wall increases with increase in mass and stiffness of self-weight of shear wall.
- Shear wall frame increases the lateral stiffness of the building, thereby reducing displacement in all storey levels.
- Shear wall are found to be most effective in increase the stiffness.

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