

COMPARATIVE ANALYSIS OF HIGH STRENGTH CONCRETE AND NORMAL STRENGTH CONCRETE COLUMNS EXPOSED TO FIRE

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Abstract - The increased usage of high strength concrete in buildings has resulted in concern regarding the behavior of such concrete under fire. In particular, spalling at certain temperatures, as identified by performing a fire test on concrete columns in laboratories, is of particular concern. This paper provides comparative analysis of structural behavior and the principal influences of high temperature in concrete at loss of compressive strength and spalling, the unintended discharge of material from a member of high strength concrete (HSC) column and normal strength concrete (NSC) columns. Though a lot of information has been gathered on both phenomena, there remains a need for a broader understanding of the response of concrete structures to different heating rates and the performance of complete concrete structures subjected to realistic fire exposures. There is a lack of information derived from large-scale tests on concrete buildings in natural fires. So we are experimenting fire test on Four Reinforced (normal strength and higher strength) concrete columns in laboratory to get actual behavior of concrete columns.

Key Words: High Strength Concrete, Normal Strength Concrete, Structural behaviour, Spalling, Fire resistance.

1. INTRODUCTION

Current concrete design Codes raise concerns about concrete spalling during fire particularly under compressive stresses and high heating rates. High strength concrete columns would be more prone for explosive spalling due to their low permeability and high brittleness. This paper represents an experimental program on the behavior of high strength concrete columns under fire. The research includes testing reinforced high strength concrete columns subjected to various loading levels and heating rates. We used to trace the structural behavior of reinforced concrete columns at elevated temperatures. A comparison is made of the fire resistance performance of High strength concrete (HSC) columns with that of normal strength concrete (NSC) columns. The factors that influence the thermal and structural behavior of HSC concrete columns under fire conditions are discussed. The results presented will generate

data on the fire resistance of high performance concrete columns and contribute to identifying the difference in behavior between HSC and NSC columns.

2. RESEARCH SIGNIFICANCE

The objective of this paper is to report the main outcomes of experimental study on the effect of loading and heating rates on explosive spalling and structural and physical behavior of high strength concrete columns and normal strength concrete column under fire. The paper includes the test methodology, main results, conclusions and measured parameters including temperatures of high strength concrete column and normal strength concrete column.

3. THE EXPERIMENTAL STUDY

3.1 Test Specimens

The tests involved main 4 reinforced concrete columns of a section 150mm x 150mm and 720 mm height. Each column was reinforced with four 12mm diameter steel bars and connected with 6 steel ties (8mm diameter) at 100 mm intervals. Ties were located more often near the ends to prevent any possible local column failure near the loading points.

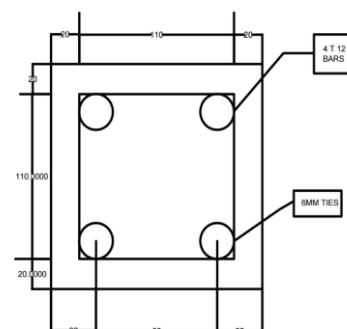


Fig -1 : HSC and NSC column after fire^[13]

We use tested Type 10 Portland cement and other all tested materials like coarse aggregate and fine aggregate, water and good quality of other cementitious material like fiber and silica fumes. The average compressive cylinder strengths of the concrete, measured 7 and 28 days after pouring and on the day of the testing. The moisture condition at the center of the column was also measured on the day of the test.

3.2 Test Apparatus

The tests were carried out by exposing the columns to heat in a furnace specially built for testing loaded columns. The furnace consists of a steel framework supported by four steel columns, with the furnace chamber inside the framework. The test furnace was designed to produce conditions, such as temperature, structural loads and heat transfer, to which a member might be exposed during a fire.

3.3 Test Conditions and Procedure

The columns were installed in the furnace to a loading head at the top and to a hydraulic jack at the bottom. For each column, the length exposed to fire was approximately 3000 mm. At high temperature, the stiffness of the unheated column ends, which is high in comparison to that of the heated portion of the column, contributes to a reduction in the column effective length. In previous studies, it was found that, for columns tested fixed at the ends; an effective length of 2000 mm represents experimental behaviour.

Concentric loads were used to test all the columns. The load intensity, defined as the ratio of the applied load to the column resistance, differentiated slightly from Columns to columns determined the influence of load on fire resistance.

The load was applied approximately 45 min before the start of the fire test and was maintained until a condition was reached at which no further increase of the axial deformation could be measured. This was selected as the initial condition for the axial deformation of the column. During the test, the column was exposed to heating controlled in such a way that the average temperature in the furnace followed, as closely as possible. It was found that the load was remained constant throughout the duration of test. The columns were considered to have failed and the tests were terminated when the hydraulic jack, which has a maximum speed of 75 mm/min, could no longer maintain the load.

3.4 RESULTS AND DISCUSSION

After testing the specimens, the temperature-time curves are to be plotted for the external surface and for various depths in concrete columns HSC1, HSC2, HSC3 and NSC1 respectively. The measured temperature in the furnace followed the standard temperature-time curve. Reason of failure of the columns is to find out.



Fig -2 : HSC and NSC column after fire^[13]

All three columns failed in compression mode or in tension mode are to be observed. In the NSC and HSC column, significant spalling occurred until the failure of the column is to be find out

The decreased fire resistance for HSC columns and NSC column can be attributed to the thermal and mechanical properties of HSC. Further, the spalling phenomenon, which resulted in the decrease in the cross-section of the column, also contributed to lowering the fire resistance in the HSC columns. It can be attributed to the type of aggregate and other materials used in the concrete mix, as explained above.

4. CONCLUSION

1. The effect of material like light weight aggregate, supplementary cementitious material such as Silica Fume and Polypropylene fibers on HSC and NSC column exposed to fire.
2. The comparative analysis between HSC and NSC columns with respect to high temperature and time.

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