

# Effect of Response Reduction Factor on R.C Building with Different Infill Material

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**Abstract** - This paper describes the actual value of response reduction factor ( $R$ ) for light weight infill material with the help of over strength, redundancy, damping and ductility. The analysis carried out by static nonlinear (pushover) analysis and this analysis is carried out by ETABS. For calculation of Response reduction factor ( $R$ ) procedure is using as per Applied Technology Council (ATC)-19 which is the product of Strength factor ( $R_s$ ), Ductility factor ( $R_\mu$ ) and Redundancy factor ( $RR$ ). After evaluating  $R$  value find out the shear forces and displacement for clay brick and light weight infill material. The study conclude that the response reduction factor is decreases when we use clay burned bricks and increases when we use light weight infill material.

**Key Words:** Response Reduction Factor, pushover analysis, light weight infill material, capacity curve.

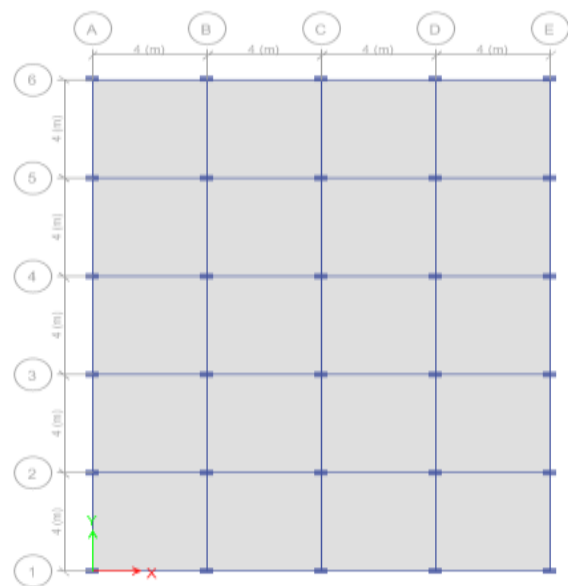
## 1. INTRODUCTION

In general design practices in India, the strength and stiffness of infill walls are ignored with the assumption of conservative design. In actual, infill walls add considerably to the strength and rigidity of the structures and their negligence will cause failure of many of multi-storeyed buildings. (Goel, 2015)

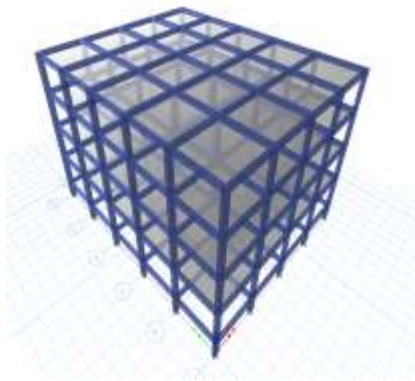
For the functional and architectural requirements Masonry walls are provided in R.C. structures. The term infilled frame is used to represent a composite structure formed by the combination of a moment resisting R.C. frame & Infill walls. The Infill walls can be of conventional clay brick (CB), concrete block or AAC block. It has been recognized that infill materials significantly affect the seismic performance of the resulting in-filled frame structures. (Goel, 2015). For seismic design of structure is indirectly based on response reduction factor ( $R$ ). Response reduction factor is defined differently in different countries for different types of structural systems.  $R$  is termed as the "response reduction factor" in the Indian standard IS 1893 and "response modification coefficient" in ASCE. In Eurocode the same factor is called "Behaviour factor" (Arunkumar 2016). According to Indian code the value of  $R$  is varies from 3 to 5 (i.e., OMRF and SMRF). In the present study, for safe economical design it is necessary to find out actual value of  $R$ .

## 1.1 METHODOLOGY

In this paper same building is used for pushover analysis. Only different is that four out of two is modeled for light weight infill material (AAC block) and remaining for normal clay burned brick. Four storied building having 5 bays in Y direction and 4 bays in X direction. To avoid effect of column sizes as well as effect of irregularity the RCC building model as column and beam section is used same throughout the building. Plastic hinges are assigned to the beam and column sections. So that the collapse mechanism is takes place. For pushover analysis the whole building is modeled as per ATC-19 (displacement control method). From pushover analysis capacity curve is getting out, with the help of that over strength factor and ductility factor calculated. All other modelling parameters are given below table



**Fig 01:** Plan view of G+4 story building considered for analysis



**Fig 02:** 3D view of G+4 story building considered for analysis

**Table -1:** Modelling parameters

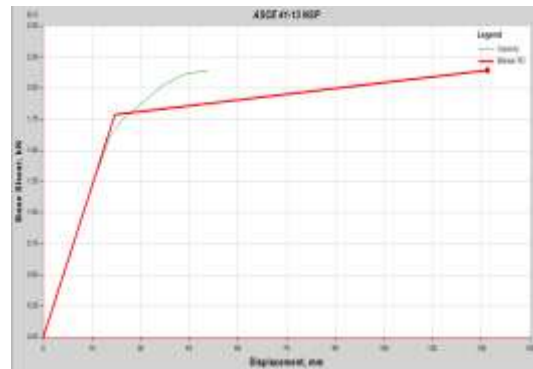
Sr. No.	DESCRIPTION	SIZE
1	Dimensions	16m X 20m
4	Spacing in X-directions	4m
5	Spacing in Y-directions	4m
6	Height of story	3m
7	Number of story	G+4
8	Height of building	13.5m
9	Materials	M-25, Fe 500
10	Beam	0.23m X 0.38m
11	Column	0.23m X 0.45m
12	Thickness of slab	0.125
13	Live load	2
14	Floor finish	1
15	Zone factor (Z)	0.16 (III)
16	Response reduction factor (R)	5 (SMRF)
17	Soil type	II
18	Importance factor (I)	1

**Load Combination:**

**Table 2:** Loading combinations as per IS 1893:2016 (part-1).

Sr. No.	Load Combinations	Case
1	Combo 1	1.5 (DL + IL)
2	Combo 2	1.2 (DL + IL+ EL)
3	Combo 2	1.5(DL + EL)
4	Combo 2	0.9 DL + 1.5 EL)

**2. Analysis and result discussion**



**Chart 01:** Pushover curve for 1st model

The above capacity curve is obtained from pushover analysis. With the help of this curve the following results have been taken.

Ultimate base shear (Vu) = 1623.51 KN

Design base shear (Vu) = 1207.27 KN

Ultimate displacement (Δmax) = 50.67 mm

Yield displacement (Δu) = 14.95 mm

$$\text{Overstrength factor (Rs)} = \frac{\text{Ultimate base shear (Vu)}}{\text{Design base shear (Vu)}}$$

$$= \frac{1623.51}{1207.27}$$

$$= 1.344$$

$$\text{Ductility reduction factor (Rμ)} = \frac{\Delta_{max}}{\Delta_u}$$

$$= \frac{50.67}{14.95}$$

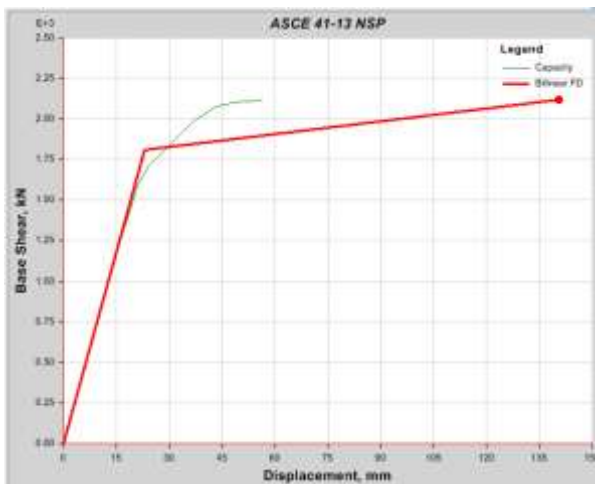
$$= 3.389$$

Redundancy factor (RR) = 1

$$\text{Response reduction factor} = R_s \times R_\mu \times R_R$$

$$= 1.344 \times 3.389 \times 1$$

$$= 4.554$$



**Chart 02:** Pushover curve for 2nd model

The above capacity curve is obtained from pushover analysis. With the help of this curve the following results have been taken.

Ultimate base shear ( $V_u$ ) = 1606.29 KN

Design base shear ( $V_u$ ) = 1171.84 KN

Ultimate displacement ( $\Delta_{max}$ ) = 56.32 mm

Yield displacement ( $\Delta_u$ ) = 14.90 mm

$$\text{Overstrength factor (Rs)} = \frac{\text{Ultimate base shear (Vu)}}{\text{Design base shear (Vu)}}$$

$$= \frac{1606.29}{1171.84}$$

$$= 1.371$$

$$\text{Ductility reduction factor (Rμ)} = \frac{\Delta_{max}}{\Delta_u}$$

$$= \frac{56.32}{14.90}$$

$$= 3.77$$

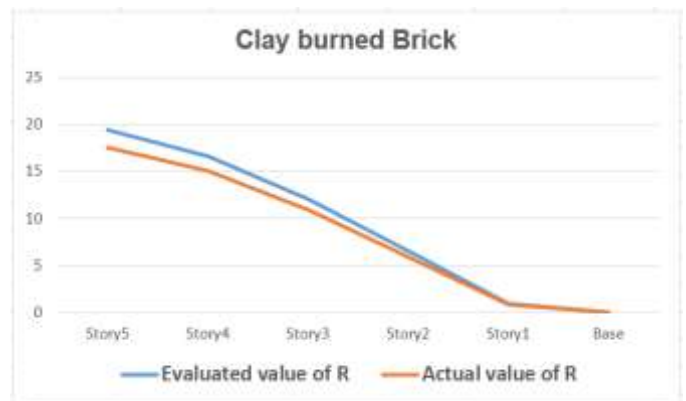
Redundancy factor (RR) = 1

$$\text{Response reduction factor} = R_s \times R_\mu \times R_R$$

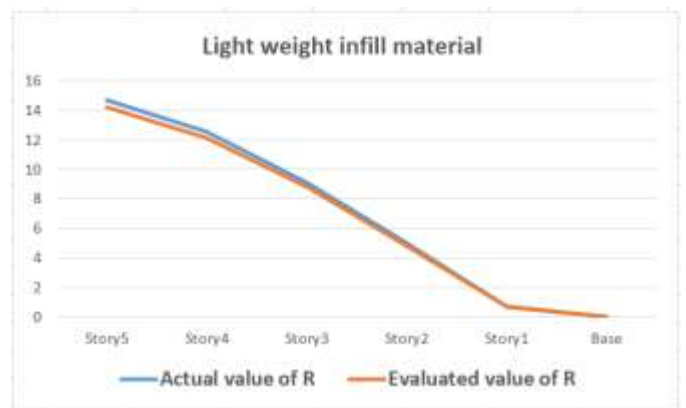
$$= 1.371 \times 3.77 \times 1$$

$$= 5.17$$

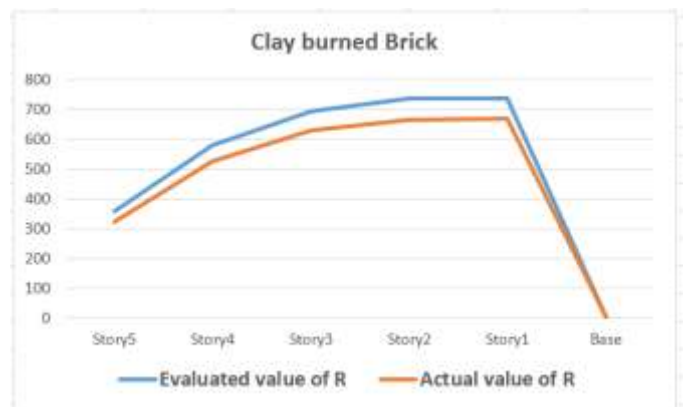
Comparison of evaluated response reduction factor with actual response reduction factor:



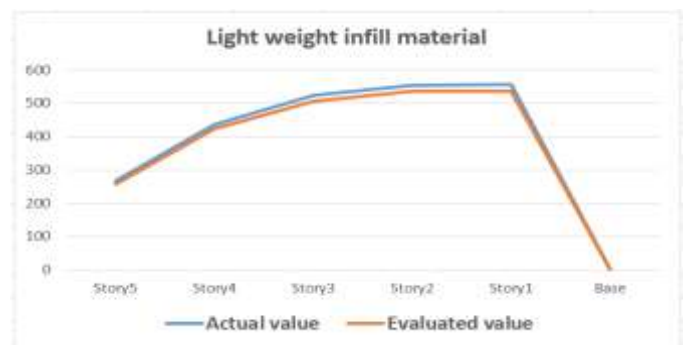
**Chart 03:** Story Displacement Vs No. of Story.



**Chart 02:** Story Displacement Vs No. of Story.



**Chart 04:** Base shear Vs No. of Story.



**Chart 05:** Base shear Vs No. of Story.

**Result and discussion**

Based on software analysis, response reduction factor, story shear and story displacement are compared.

**Table -3:** Comparison of clay burned brick and light weight infill material (AAC block).

	Clay burned brick		Light weight infill material	
	Evaluated value of R	Actual value of R	Evaluated value of R	Actual value of R
Response reduction factor R	4.55	5	5.17	5
story shear	737.05	667.62	537.71	555.99
story displacement	19.41	17.60	14.154	14.63

**3. CONCLUSIONS**

From the above analysis it is clear that the response reduction factor is decreases when we use clay burned bricks and increases when we use light weight infill material.

It is more dangerous when the value of response reduction factor R decreases. Because the value of base shear and story displacement are increases.

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