# Estimating the Vehicle Operating Cost through Railway Over Bridge 

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#### Abstract

Understanding the vehicle working expense is or vehicle operating cost is fundamental to sound arranging and administration of street ventures. While the framework costs borne by streets offices are generous, the expenses borne by streets clients are much more noteworthy. To qualify these connections, World Bank started a cooperative worldwide examination which prompted the vehicle working costs connections created in this investigation, and presents these in a little simple to utilize PC program which can be utilized freely of the bigger model.


The HDM-VOC program predicts the different parts of vehicle working costs in light of street and vehicle qualities and unit costs in a free stream movement condition. The vehicle working expenses was additionally evaluated by Indian Road Congress parameters. IRC gives a few conditions to assess the VOC for various kinds of vehicles like autos, LCV, transport and so forth in IRC SP 30:2009. This paper is planned to evaluate the vehicle working expense of ROB at LC-70, Sitapura, Jaipur. For this reason world bank investigation of VOC and IRC SP 30:2009 are utilized. By the utilization of these examinations computed the VOC with geometric outline components of ROB at various velocities. These rates are 100 and 120 kmph .

Key Words: Vehicle operating cost (VOC), geometric elements of road, LC-70 (Lacation), HDM-VOC (Software used)

## Introduction

The Jaipur Development Authority (JDA) has approved the design of railway over bridge (ROB) to be built on JaipurSawai Madhopur railway track in Sitapura area. This would be the first Y-shaped ROB in the city. The estimated cost of the ROB is Rs 116 crore.

The length of the over bridge will be 925 meters. It will start ahead of Mahatma Gandhi crossing.

It will be a six-lane ROB and its one wing will move in the north direction towards the slip lane connecting Sitapura.
located between Sanganer and Shivdaspura Stations on Jaipur-Sawai Madhopur BG Railway Line.

## Sight Distance for Various Speeds

| Speed <br> $(\mathrm{km} / \mathrm{h})$ | Sight distance (meter) |  |  |
| :---: | :---: | :---: | :---: |
|  | Stopping | intermediate | Overtaking |
| 20 | 20 | 40 | - |
| 25 | 25 | 50 | - |
| 30 | 30 | 60 | - |
| 35 | 40 | 80 | - |
| 40 | 45 | 90 | 165 |
| 50 | 60 | 120 | 235 |
| 60 | 80 | 160 | 300 |
| 65 | 90 | 180 | 340 |
| 80 | 120 | 240 | 470 |
| 100 | 180 | 360 | 640 |
| 120 | 240 | 480 | 720 |

(Source: irc.gov.sp.023:1993)

## Design table of Summit Curve (Tonk Road Side) for SSD

|  | 4. |  | \& |  | 4.n <br> lagn <br> dimm <br> (A) | 1 | (1) | Mystrpint <br> anter <br> anventive <br> BMCpirit | Nuthe yigt <br> pint M. | 315 | SIL | $\begin{aligned} & \mathrm{Rlif} \\ & \mathrm{BIC} \\ & \text { pint } \end{aligned}$ |  | $\begin{aligned} & \text { Raf } \\ & \text { wiget } \\ & \text { pint } \\ & \text { want } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 100 | 003 | 103 | 10 | 1266 | 20 | 13930 | 8896 | 20 | $3 / 7 \%$ | $\begin{aligned} & 68 \\ & 26 \\ & 26 \end{aligned}$ | $\begin{aligned} & 88 \\ & 25 \end{aligned}$ | 32215 | 3645 | 13:3\% |
| 10. | 003 | 1038 | 2-W | 346 | 130 | 12112 | 10.600 | 330 | S/17 | $\begin{aligned} & 68 \\ & 25 \\ & 25 \end{aligned}$ | $\begin{aligned} & 48 \\ & 25 \end{aligned}$ | 32215 | 3/360 | 36188 |

## Design table of Summit Curve (Tonk Road Side) for OSD

## Project Location

This ROB will ease traffic on Road connection Sitapura Industrial area to Jaipur - Tonk Road (NH-12). LC No. 70 is

| $\begin{aligned} & \text { Dsign } \\ & \text { Spud } \\ & (\mathrm{m}(\mathrm{~m}) \end{aligned}$ | It | 12 | V | S |  |  |  |  | Ruthe <br> mghest <br> pint <br> M | BICS | NCS | LabBlcpin | Rofligherpion angradine | $\begin{array}{\|c\|} \text { Redof } \\ \text { hipget } \\ \text { pinite } \\ \text { curve } \end{array}$ | Stations | Chainage from BVC | RL of Points on Grade Line | Ordinates B/W Curve and Grade Line | RL of Station on Curve |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | 1 |  |  |  |  |  |  |  | 0 | 364.2 | 0 | 364.2 |
|  |  |  |  |  |  |  | 2 |  |  |  |  |  |  |  | 10 | 363.95 | 0.006578947 | 363.9434211 |
|  |  |  |  |  |  |  |  |  |  |  | 3 |  |  |  | 20 | 363.7 | 0.026315789 | 363.6736842 |
| 100 | 0033 | 0 | 0033 | 60 | 98909 | $10000_{61}^{660}$ |  | 600 300 | 100 | 165 | 482 | 2 | 322015 | 335015 | 37858 | 4 | 30 | 363.45 | 0.059210526 | 363.3907895 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 5 | 40 | 363.2 | 0.105263158 | 363.0947368 |
| 120 | 0033 | 0 | 0033 | 720 | 114909 | $11500_{69}^{690}$ |  |  | 1150 | 18975 | (4825 | 1782 | 36205 | 4000 | 3810: | 6 | 50 | 362.95 | 0.164473684 | 362.7855263 |

Design table of Summit Curve (Mahatma Gandhi Hospital Side) for SSD

| Design Speed (m/s) | n1 | n2 | N | S |  |  | a | R | Highest pointon the curveXD from Bics point | RL of the highest point (I) | B/CS | EVCS R | RL of BVCpoing | RL of highest poin on Grade Line | RL of highest pointit curve |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 100 | 0.033 | 0 | 0.033 | 640 | 989,09 | 1000 | $\begin{array}{\|c\|} \hline 660 \\ 6.1 \\ \hline \end{array}$ | 3030 3.0 | 1000 | 16.5 | 64825 | 16482. | 36205 | 395.05 | 378.55 |
| 120 | 0.033 | 0 | 0.033 | 720 | 1149,09 | 1150 | $\begin{array}{\|c\|} \hline 6969 \\ 6.9 \\ \hline \end{array}$ | $\begin{array}{\|c\|} 3484 \\ 8.5 \\ \hline \end{array}$ | 1150 | 18975 | 64825 | 17982.2. | 362.05 | 400,0 | 381.03 |

## Design table of Summit Curve (Mahatma Gandhi Hospital Side) for OSD

| $\left\|\begin{array}{l} \text { Dexigy } \\ \text { speed } \\ \text { (m/s } \end{array}\right\|$ | 121 | 12 | V | S | Calultad <br> enght of umy <br> (L) | Alopterilenti of curve (L) | 1 | R | Highest pointon the curve whom som BRE point | RL ofthe highest point(I) | B/CS |  | RL ofBVCpoint | $\begin{aligned} & \text { RL of igigles } \\ & \text { point on } \\ & \text { Grade Line } \end{aligned}$ | $\begin{gathered} \text { RL of highestpoint } \\ \text { curve } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 100 | 0 | .0.025 | 0.025 | 640 | 1104 | 1105 |  |  | 1 | 0 | 934,77 | 2039, ${ }^{17}$ | 367, 2 | 364.2 | 367, 2 |
| 120 | 0 | $\cdot 0.025$ | 0.025 | 720 | 1264 | 1265 |  | $50600$ | 0 | 0 | 934.77 | 2199, ${ }^{\prime \prime}$ | 364.2 | 364.2 | 364.2 |

Summit curve I (Tonk Road Side) at 100 kmph

| Stations | Chainage <br> from BVC | RL of Points <br> on Grade <br> Line | Ordinates B/W <br> Curve and Grade <br> Line | RL of Station on <br> Curve |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 362.055 | 0 | 362.055 |
| 2 | 10 | 362.385 | 0.007173915 | 362.3778261 |
| 3 | 20 | 362.715 | 0.02869566 | 362.6863043 |
| 4 | 30 | 363.045 | 0.064565236 | 362.9804348 |
| 5 | 40 | 363.375 | 0.114782641 | 363.2602174 |
| 6 | 50 | 363.705 | 0.179347877 | 363.5256521 |

## WHEN DESIGN SPEED IS 100 KMPH



Summit curve II (Hospital Side) at 100 kmph

WHEN DESIGN SPEED IS $\mathbf{1 0 0} \mathbf{K M P H}$


Summit curve I (Tonk Road Side) at 120 kmph

| Stations | Chainage <br> from BVG | RL of <br> Points on <br> Grade Line | Ordinates B/W <br> Curve and Grade <br> Line | RL of Station on <br> Curve |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 362.055 | 0 | 362.055 |
| 2 | 10 | 362.385 | 0.004714286 | 362.3802857 |
| 3 | 20 | 362.715 | 0.018857144 | 362.6961429 |
| 4 | 30 | 363.045 | 0.042428574 | 363.0025714 |
| 5 | 40 | 363.375 | 0.075428576 | 363.2995714 |
| 6 | 50 | 363.705 | 0.11785715 | 363.5871429 |



Summit curve II (Hospital Side) at $120 \mathbf{~ k m p h}$

| Stations | Chainage <br> from <br> BVC | RL of <br> Points on <br> Grade Line | Ordinates B/W <br> Curve and <br> Grade Line | RL of Station on <br> Curve |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 364.2 | 0 | 364.2 |
| 2 | 10 | 363.95 | 0.001131222 | 363.9488688 |
| 3 | 20 | 363.7 | 0.004524887 | 363.6954751 |
| 4 | 30 | 363.45 | 0.010180995 | 363.439819 |
| 5 | 40 | 363.2 | 0.018099548 | 363.1819005 |
| 6 | 50 | 362.95 | 0.028280543 | 362.9217195 |



Design Table for Valley Curve (Tonk Road Side)

| $\begin{gathered} \text { Desi } \\ \text { gn } \\ \text { Spe } \\ \text { ed } \\ (\mathrm{m} / \\ \mathrm{s}) \end{gathered}$ | n1 | n2 | 2 N |  | $\begin{gathered} \text { Calc } \\ \text { ulate } \\ \text { d } \\ \text { S lengt } \\ \text { h of } \\ \text { curv } \\ \text { e (L) } \end{gathered}$ | Adop ted lengt $h$ of curv e (L) | a |  | High est point on the curv <br> (X) from BVCS point | $\begin{gathered} \text { RL } \\ \text { of } \\ \text { the } \\ \text { high } \\ \text { est } \\ \text { poin } \\ \text { por } \end{gathered}$ | BV S | S | $\begin{gathered} \text { RL } \\ \text { of } \\ \text { BVC } \\ \text { poin } \\ t \end{gathered}$ | RL of high est poin ton Grad e Line | $\begin{gathered} \text { RL } \\ \text { of } \\ \text { of } \\ \text { est } \\ \text { est } \\ \text { poin } \\ \text { dat } \\ \text { cur } \\ \text { cut } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} 10 \\ 0 \end{gathered}$ | $\begin{array}{\|c\|} \hline- \\ 0.0 \\ 00 \\ 79 \\ \hline \end{array}$ | $\left\|\begin{array}{c} 0 . \\ 03 \\ 3 \end{array}\right\|$ | $\begin{array}{c\|c} 0 . \\ 3 \\ 33 \\ 4 \end{array}$ | $\begin{array}{c\|c} 18 \\ 3 & 18 \\ 0 \end{array}$ | $\begin{array}{c\|c} 18 & 131 . \\ 0 & 19 \end{array}$ | 140 | $\begin{aligned} & 821 \\ & 3.5 \end{aligned}$ | $\begin{gathered} 410 \\ 6.7 \\ 7 \end{gathered}$ | $\left.\begin{gathered} 0 \\ 3.40 \\ 20 \end{gathered} \right\rvert\,$ | $\left\lvert\, \begin{gathered} 0.0 \\ 014 \\ 09 \end{gathered}\right.$ | 425 .64 | $\begin{aligned} & 565 \\ & .64 \end{aligned}$ | 355 <br> .57 | . 68 | 355 .68 |
| $\begin{gathered} 12 \\ 0 \end{gathered}$ | $\begin{aligned} & 0.0 \\ & 00 \\ & 79 \end{aligned}$ | $\left\|\begin{array}{c} 0 . \\ 03 \\ 3 \end{array}\right\|$ | $\begin{array}{c\|c} 0 . \\ 3 \\ 33 \\ 4 \end{array}$ | $\begin{array}{c\|c} \mathbf{3}_{2}^{24} \\ \hline \end{array}$ | $\begin{array}{c\|c} \hline 4 & 189 . \\ 0 & 59 \end{array}$ | 200 | $\begin{array}{\|c} 117 \\ 33 . \\ 64 \end{array}$ | $\begin{gathered} 586 \\ 6.8 \\ 2 \end{gathered}$ | $\mathfrak{c \| c} \left\lvert\, \begin{gathered} - \\ 4.86 \\ 00 \end{gathered}\right.$ | $\left(\begin{array}{c} 0.0 \\ 020 \\ 13 \end{array}\right.$ | 425 <br> .64 | 625 | 355 <br> .57 | 355 | 355 |

Design Table for Valley Curve (Mahatma Gandhi Hospital Side)

| $\begin{gathered} \text { Des } \\ \text { gn } \\ \text { Spe } \\ \text { ed } \\ (\mathrm{m} / \\ \mathrm{s}) \end{gathered}$ | n1 | n2 | N S | $\begin{gathered} \text { Calc } \\ \text { cale } \\ \text { d } \\ \text { S lengt } \\ \text { h of } \\ \text { curv } \\ \text { e (L) } \end{gathered}$ | $\begin{gathered} \text { Adop } \\ \text { ted } \\ \text { lengt } \\ \text { h of } \\ \text { curv } \\ e(\mathrm{~L}) \end{gathered}$ | a | R | High est poin on the curv e(X) from BVCS point | RL of the high est poin $t(Y)$ | BVC | S | $\begin{gathered} \text { RL } \\ \text { Cof } \\ \text { BVC } \\ \text { poin } \\ \text { t } \end{gathered}$ | RL of high est poin t on Grad Line | RL of high est poin t at cury e |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} 10 \\ 0 \end{gathered}$ | $\begin{aligned} & 0.0 \\ & 25 \end{aligned}$ | $\begin{gathered} - \\ 0 . \\ 00 \\ 2 \\ \hline \end{gathered}$ | $\begin{array}{\|c\|c} 0 . & 18 \\ 02 & 18 \\ 3 & 0 \\ \hline \end{array}$ | $\begin{array}{c\|c} 18 & 15 . \\ 0 & 40 \end{array}$ | 60 | $\begin{gathered} 530 \\ 1.5 \end{gathered}$ | $\begin{gathered} 265 \\ 0.7 \\ 6 \end{gathered}$ | $\begin{gathered} 54.8 \\ 1 \end{gathered}$ | 0.5 | . 334 | 394 | . 354 | 353 | $\begin{gathered} 35 \\ 3.9 \\ 6 \end{gathered}$ |
| $\begin{aligned} & 12 \\ & 0 \end{aligned}$ | $\begin{aligned} & 2.0 \\ & 25 \end{aligned}$ | 0. | $\begin{array}{\|l\|l} \hline 0 . & 24 \\ 02 & 2 \\ 3 & 0 \\ \hline \end{array}$ | $\begin{array}{l\|l} 42 . \\ 6 & 42 \end{array}$ | 0 | $\begin{aligned} & 706 \\ & 8.7 \end{aligned}$ | $\begin{aligned} & 353 \\ & 4.3 \\ & 5 \end{aligned}$ | 73.0 8 | 0.7 | 334 | 414 | 354 | . 352 | $\begin{aligned} & 35 \\ & 3.7 \end{aligned}$ |

## Valley curve I (Tonk Road Side) at 100 kmph

| Stations | Chainage <br> from <br> BVC | RL of Points <br> on Grade <br> Line | Ordinates B/W <br> Curve and <br> Grade Line | RL of Station <br> on Curve |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 355.57 | 0 | 355.57 |
| 2 | 10 | 355.5621 | 0.012175001 | 355.574275 |


| 3 | 20 | 355.5542 | 0.048700002 | 355.6029 |
| :---: | :---: | :---: | :---: | :---: |
| 4 | 30 | 355.5463 | 0.109575005 | 355.655875 |
| 5 | 40 | 355.5384 | 0.194800009 | 355.7332 |
| 6 | 50 | 355.5305 | 0.304375013 | 355.834875 |

WHEN DESIGN SPEED IS 100 KMPH


## Valley curve II (Hospital Side) at 100 kmph



## Valley curve I (Tonk Road Side) at 120 kmph

| Stations | Chainage <br> from BV | RL of <br> Points on <br> Grade Line | Ordinates B/W <br> Curve and <br> Grade Line | RL of Station on <br> Curve |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 355.57 | 0 | 355.57 |
| 2 | 10 | 355.5621 | 0.008522505 | 355.5706225 |
| 3 | 20 | 355.5542 | 0.034090018 | 355.58829 |
| 4 | 30 | 355.5463 | 0.076702541 | 355.6230025 |
| 5 | 40 | 355.5384 | 0.136360072 | 355.6747601 |



## Valley curve II (Hospital Side) at 120 kmph

| Stations | Chainage <br> from BV | RL of <br> Points on <br> Grade Line | Ordinates B/W <br> Curve and <br> Grade Line | RL of Station on <br> Curve |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 0 | 354.64 | 0 | 354.64 |
| 2 | 5 | 354.515 | 0.003536718 | 354.5185367 |
| 3 | 10 | 354.39 | 0.014146873 | 354.4041469 |
| 4 | 15 | 354.265 | 0.031830464 | 354.2968305 |
| 5 | 20 | 354.14 | 0.056587491 | 354.1965875 |
| 6 | 25 | 354.015 | 0.088417955 | 354.103418 |



Minimum Curve Radius for Different Design Speeds

| Design Speed |  <br> Rolling <br> Terrain(m) | Mountainous and Steep Terrain |  |
| :---: | :---: | :---: | :---: |
|  |  | Snow Bound Area $(\mathrm{m})$ | Non-snow Bound Area (m) |
| 20 | 15 | 15 | 14 |
| 25 | 23 | 23 | 20 |
| 30 | 33 | 33 | 30 |
| 35 | 45 | 45 | 40 |
| 40 | 60 | 60 | 50 |
| 50 | 90 | 90 | 80 |
| 65 | 155 | Speed not applicable |  |
| 80 | 230 |  |  |
| 100 | 360 |  |  |
| 120 | 450 |  |  |

(Source: irc 38:1988)

## Length of Transition Curve as Per Radius of Curvature and Design Speed

Transition Lengths ( m ) for Plain and Rolling Terrain

| Curve Radius (m) | $\begin{aligned} & 100 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ | $\begin{aligned} & 80 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ | $\begin{aligned} & 65 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ | $\begin{aligned} & 50 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ | $\begin{aligned} & 40 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ | $\begin{aligned} & 35 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ | $\begin{aligned} & 30 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ | $\begin{aligned} & 25 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ | $\begin{aligned} & 20 \\ & \mathrm{~km} \\ & / \mathrm{h} \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |
| 20 | - | - | - |  |  |  | - | - | 55 |
| 40 | - |  |  |  |  |  | 60 | 45 | 30 |
| 80 | - | - | - | - | 55 | 45 | 30 | 25 | 15 |
| 100 | - | - | - | 70 | 45 | 35 | 25 | 20 | - |


| 125 | - | - | - | 55 | 35 | 30 | 20 | 15 | - |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 155 | - | - | 80 | - | - | - | - | - | - |
| 200 | - | - | 60 | 35 | 20 | 15 | 15 | - | - |
| 250 | - | 90 | 50 | 30 | 20 | - | - | - | - |
| 300 | - | 75 | 40 | 25 | - | - | - | - | - |
| 350 | 130 | 60 | 35 | 20 | - | - | - | - | - |
| 360 | 130 | - | - | - | - | - | - | - | - |
| 400 | 115 | 55 | 30 | 20 | - | - | - | - | - |

(Source: IRC 38:1988)
Design Table of Horizontal Curve (Tonk Road Side)

| S.No |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\cdot$ | Design <br> Speed <br> $(\mathrm{km} / \mathrm{h}$ <br> $)$ | Rc <br> $(\mathrm{m})$ | e | e <br> take <br> n | f | PC | PT | LCLong <br> Chord <br> Deviatio <br> n Angle <br> $(\Delta \mathrm{S})$ |  |
| 1 | 120 | 515.5 <br> 2 | 0.12 <br> 4 | 0.07 | 0.14 <br> 9 | 665.2 <br> 5 | 785.2 <br> 5 | 120 | 13.37 |
| 2 | 100 | 358 | 0.12 <br> 4 | 0.07 | 0.14 <br> 9 | 665.2 <br> 5 | 785.2 <br> 5 | 120 | 19.18 |

Design Table of Horizontal Curve (Tonk Road Side)

| Ls | $\begin{gathered} \text { Adop } \\ \text { ted } \\ \text { Ls } \end{gathered}$ | $\theta \mathrm{s}$ | Shif $t$ for unit leng th of $\theta$ |  | $\begin{gathered} \text { K } \\ \text { for } \\ \text { unit } \\ \text { leng } \\ \text { lh } \end{gathered}$ | K | $\begin{gathered} (\Delta s \\ ) \end{gathered}$ | ( $\Delta$ ) | Ts | Es | $\begin{gathered} (\mathrm{Lc} \\ \mathrm{J} \end{gathered}$ | L |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 75. | 1 | 7.2 | 0.01 | 1.3 | 0.53 | 69. | 13. | 27. | 2303 | 1776 | 120 | 250. |
| 41 |  | 2 | 05 | 65 | 51 | 56 | 37 | 82 | . 03 | . 98 | . 2 | 23 |
| 75. | 130 | 10. | 0.01 | 1.9 | 0.49 | 64. | 19. | 39. | 856. | 511. | 119 | 249. |
| 41 | 130 | 40 | 51 | 63 | 95 | 93 | 18 | 98 | 66 | 7 | . 7 | 78 |

## Design Table of Horizontal Curve (Hospital Side)

| S.No <br> $\cdot$ | Design <br> Speed <br> $(\mathrm{km} / \mathrm{h}$ <br> $)$ | Rc <br> $(\mathrm{m})$ | e | e <br> take <br> n | f | PC | PT | LC <br> (Long <br> Chord <br> ) | Deviatio <br> n Angle <br> $(\Delta \mathrm{S})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 120 | 515.5 <br> 2 | 0.12 <br> 4 | 0.07 | 0.14 <br> 9 | 870.4 <br> 3 | 966.4 <br> 3 | 96 | 10.68 |
| 2 | 100 | 358 | 0.12 <br> 4 | 0.07 | 0.14 <br> 9 | 870.4 <br> 3 | 966.4 <br> 3 | 96 | 15.32 |

## Design Table of Horizontal Curve (Hospital Side)



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All the geometric analysis (tables \& charts showing total horizontal \& vertical profile of the ROB LC-70) is done by using the all codes from Indian Road Congress such as:

1. IRC $86: 1983$ is used to calculate geometric design standards for urban roads in Plains.
2. IRC SP 30:2009 is used for manual economic evaluation of highway projects in India
3. IRC $38: 1988$ gives guidelines for design of horizontal curves for highways and design tables
4. IRC SP 023:1993 is used to calculate the vertical curves for highways.

## VOC THROUGH ROB BY WORLD BANK (HDM-VOC)

The World Bank started a community worldwide examination which prompted the VOC connections created in this investigation. These connection show in alittle simple to utilize PC program, this can be utilized freely of the bigger model. The HDM-VOC program predicts the different parts of VOC in respect to vehicle \& road qualities and unit cost in a free stream movement condition. Calculations are accommodated ten vehicle writes running from little auto to articulated truck, and process speed, physical amount devoured, and add up to working expenses.

## APPROACH OF ANALYSIS (HDM-VOC RELATIONSHIPS)

It works into steps stated below:

1. Mean operating speed of vehicle calculated.
2. Calculate amount of resources used per 1000 vehicle-km for the components as
$>$ Fuel consumption
$>$ Lubricant consumption
$>$ Tire wear
$>$ Crew time
> Passenger time
> Cargo holding
> Maintenance labor
> Maintenance parts
$>$ Depreciation
$>$ Interest
$>$ Overhead
3. Apply unit costs to the resource consumptions amounts.
4. Sum the operating cost for each component \& calculate the total VOC per 1000 vehicle-km.

## INPUTS FOR HDM-VOC PROGRAM

1. Surface Type: The model gives two choices to street surface write: (i) Paved and (ii) Unpaved. Enter 1 to choose a cleared street, and 0 to choose an unpaved street.
2. Roughness: The street unpleasantness is characterized as the deviation of a surface from a genuine planer surface with attributes that influence vehicle progression, ride
quality, dynamic burdens and seepage. Enter the normal street unpleasantness in IRI units (International roughness index, in $\mathrm{m} / \mathrm{km}$ ).

| Quantitative <br> Evaluation | Roughness IRI (m/km) |  |
| :---: | :---: | :---: |
|  | Paved Road | Unpaved Road |
| Smooth | 2 | 4 |
| Reasonably <br> smooth | 4 | 8 |
| Medium rough | 6 | 12 |
| Rough | 8 | 15 |
| Very rough | 10 | 20 |

(Source: World Bank Technical Paper No. 234)
3. Vertical Profile: Travel on a road can be understood in three different types between two points as A and B. These are: (i) One-way travel from A to B (ii) One-way travel from $B$ to $A$ (iii) Round trip travel either from $A$ to $B$ and back to $A$, or from B to A and back to B.
Take after the means underneath to process the vertical geometric totals from a definite geometric profile:
A. Start with an itemized vertical profile.
B. Divide the roadway into segments with peaks and trough as limit focuses. Decide the lengths (ls) and normal slopes (as a group and with sighns held) of the segments (gs) and shape a forbidden profile of vertical geometry.
C. Determine the 'positive gradient (ps) of each section:
If the gradient of sections is positive, i.e., $g_{s} \geq 0$, then:

$$
\mathrm{p}_{\mathrm{s}}=\mathrm{g}_{\mathrm{s}} .
$$

If the gradient of sections is negative, i.e., $g_{s}<0$, then:

$$
\mathrm{p}_{\mathrm{s}}=0
$$

D. Determine the negative gradient $\left(n_{s}\right)$ of each section:
If the gradient of sections is positive, i.e., $g_{s} \geq 0$, then:

$$
\mathrm{n}_{\mathrm{s}}=0
$$

If the gradient of section $s$ is negative, i.e., gs < 0 , then:
$n_{s}=|g s|$, where $|g s|$ is the absolute value of $g_{s}$
E. Determine the 'rise' of each section. Multiply length and positive gradient to get $\mathrm{pl}_{\mathrm{s}}$ :

$$
\mathrm{Pl}_{\mathrm{s}}=\mathrm{Ps}_{\mathrm{s}}
$$

F. Determine the 'fall' of each section. Multiply length and negative gradient to get $\mathrm{nl}_{\mathrm{s}}$ :

$$
\mathrm{nl}_{\mathrm{s}}=\mathrm{n}_{\mathrm{s}} \mathrm{l}_{\mathrm{s}}
$$

G. Specify the segments with positive gradient (uphill travel). Enter the length ls of the section if the section has a positive gradient; enter zero if the section has a negative gradient:

$$
\begin{array}{ll}
P_{s}=l_{S} & \text { if } g s \geq 0 \\
P_{s}=0 & \text { if } g s<0
\end{array}
$$

H. Form the totals of columns to get $\mathrm{L}, \mathrm{PL}, \mathrm{NL}$ and $P$, respectively.
I. Compute the average vertical geometric characteristics.
The recommended range for positive gradient (PG) and negative gradient ( NG ) is from 0 to 12 percent. The range for the proportion of uphill travel (LP) is from 0 to 100 percent.

## Computation of Vertical Aggregates for 100 kmph

\(\left.$$
\begin{array}{|c|c|c|c|c|c|c|c|}\hline \begin{array}{c}\text { Sectio } \\
\mathrm{n}\end{array} & \begin{array}{c}\text { Lengt } \\
\mathrm{h} \\
(\mathrm{m})\end{array} & \begin{array}{c}\text { Gradient } \\
\text { (Fraction) })\end{array} & \begin{array}{c}\text { Positive } \\
\text { Gradient } \\
\text { (Fractio } \\
\mathrm{n})\end{array}\end{array}
$$ $$
\begin{array}{c}\text { Negative } \\
\text { Gradient } \\
\text { (Fraction) }\end{array}
$$ ~ \begin{array}{c}Rise <br>

(\mathrm{m})\end{array}\right)\) Fall (m) | Uphill |
| :---: |
| Travel |
| $(\mathrm{m})$ |$|$

AVERAGE POSITIVE GRADIENT (PG) $=(\mathrm{PL} / \mathrm{P}) * 100=3.33 \%$
AVERAGE NEGATIVE GRADIENT (NG)= [NL/(L-P)]*100=0.747\%
AVERAGE UPHILL TRAVEL= $(\mathrm{P} / \mathrm{L}) * 100=29.84 \%$
$\mathrm{RF}=\{(365.85-355.57)+(364.2-353.96)\} / .910=22.55 \mathrm{~m} / \mathrm{Km}$
Computations of Vertical Aggregates for 120 kmph

| Sectio <br> n | Lengt <br> h <br> $(\mathrm{m})$ | Gradient <br> (Fraction) | Positive <br> Gradient <br> (Fractio <br> $\mathrm{n})$ | Negative <br> Gradient <br> (Fraction) | Rise (m) Fall (m) | Uphill <br> Travel <br> $(\mathrm{m})$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 10 <br> 0 | - <br> 0.0007 <br> 9 | 0 | 0.0007 <br> 9 | 0 | 0.079 | 0 |
| 2 | 10 <br> 0 | 0.0333 | 0.033 <br> 3 | 0 | 3.33 | 0 | 10 <br> 0 |
| 3 | 17 <br> 5 | 0.0333 | 0.033 <br> 3 | 0 | 5.827 <br> 5 | 0 | 17 <br> 5 |
| 4 | 17 <br> 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5 | 15 <br> 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| 6 | 15 <br> 5 | -0.025 | 0 | 0.025 | 0 | 3.875 | 0 |
| 7 | 40 | -0.025 | 0 | 0.025 | 0 | 1 | 0 |


|  |  | - |  | 0.0023 |  | 0.094 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 40 | 0.0023 | 0 | 65 |  | 6 | 0 |
|  | $\mathbf{L =}$ |  |  |  | PL= | NL= | P= |
|  | $\mathbf{9 4}$ |  |  |  | $\mathbf{9 . 1 5 7}$ | $\mathbf{5 . 0 4 8}$ | $\mathbf{2 7}$ |
|  | $\mathbf{0}$ |  |  |  | $\mathbf{5}$ | $\mathbf{6}$ | $\mathbf{5}$ |

AVERAGE POSITIVE GRADIENT (PG)= (PL/P)*100=3.33\%
AVERAGE NEGATIVE GRADIENT (NG)= [NL/(L-P)]*100=0.765\% AVERAGE UPHILL TRAVEL= $(\mathrm{P} / \mathrm{L}) * 100=29.25 \%$
$\mathrm{RF}=\{(367.83-355.57)+(364.2-353.74)\} / .910=24.97 \mathrm{~m} / \mathrm{Km}$

## Average Horizontal Geometric Characteristics

| Average |  | One way Trip |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Geometric <br> Characterist <br> ics | Symbol | A to B | B to A | Round- trip |
| Average <br> Curvature | C | K/L | K/L | K/L |
| Average <br> Super <br> elevation | SP | S/L | S/L | S/L |

(Source: World Bank Technical Paper No. 234)

Computations of Horizontal Aggregates for Design Speed 100 kmph

| Cur <br> vy <br> Sec <br> tion | Len <br> gth <br> $(\mathrm{m})$ | Radi <br> us of <br> Curv <br> ature <br> $(\mathrm{m})$ | Curvat <br> ure <br> $(\mathrm{deg} / \mathrm{k}$ <br> $\mathrm{m})$ | Supe <br> r <br> Elev <br> ation | $\mathrm{cl}_{\mathrm{s}}$ | $\mathrm{sl}_{\mathrm{s}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 24 | 360 | 159.23 <br> 9.8 | 0.07 | 39777. <br> 07006 | 17. <br> 486 |
| 2 | 22 | 360 | 15988 | 5.23 | 0.07 | 35939. <br> 49045 |
|  | 5.7 |  | 56688 | 799 |  |  |
|  |  |  |  |  | $\mathbf{7 5 7 1 6 .}$ | $\mathbf{3 3 .}$ <br> $\mathbf{2 8}$ <br> $\mathbf{5}$ |

ROAD LENGTH $=\mathbf{9 1 0 m}$
HORIZONTAL CURVATURE $=75716.56051 / 910=$
83.20

SUPERELEVATION $=33.285 / 910=\mathbf{0 . 0 3 6}$

Computations of Horizontal Aggregates for Design Speed 120 kmph

| Cur <br> vy <br> Sect <br> ion | Len <br> gth <br> $(\mathrm{m})$ | Radiu <br> s of <br> Curva <br> ture <br> $(\mathrm{m})$ | Curvatu <br> re <br> $(\mathrm{deg} / \mathrm{k}$ <br> $\mathrm{m})$ | Supe <br> r | Eleva <br> tion | $\mathrm{cl}_{\mathrm{s}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | $\mathrm{sl}_{\mathrm{s}}$.

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```
HORIZONTAL CURVATURE \(=52496.32533 / 910=57.69\) SUPERELEVATION \(=33.334 / 910=\mathbf{0 . 0 3 7}\)
```

Altitude of Terrain: The model uses height of landscape (the normal rise of street over the mean ocean level, in meters) to process the air protection from the vehicle movement. The prescribed range for elevation (AL) is from 0 to 5000 meters.

Effective Number of Lanes: The model gives two choices to the powerful no. of paths: (i) one lane (ii) more than one lane. Enter 1 to choose the solitary path (lane) street, and 0 to choose an in excess of one path street.

Vehicle Type: Any type of vehicle can be selected now. We are selecting small car for our dissertation.

Desired Speed: The coveted speed requirement (VDESIR) is the coveted vehicle speed without the impact of street seriousness factor. On a straight, level and smooth street, despite the fact that the driving, braking, bend and ride seriousness speed imperatives don't exist, the vehicle still does not regularly go at the speed managed by its own particular greatest or even utilized power.

## Tire Wear Information:

The tire wear taken \& uses by model as following:
$>$ Number of tires per vehicle
> Wearable volume of rubber per tire ( $\mathrm{dm}^{3}$ )
$>$ Retreading cost per new tire cost ratio (fraction)
$>$ Maximum number of recaps
$>$ Constant term of tread wear model $\left(\mathrm{dm}^{3} / \mathrm{m}\right)$
Wear coefficient of tread wear model ( $10-3 \mathrm{dm}^{3} / \mathrm{kJ}$ )

## Recommended Wearable Volume of Rubber per Tyre

| Vehicle | Recommended Wearable Volume of <br> Rubber per Tyre ( $\mathbf{d m}^{\mathbf{3}}$ ) |
| :---: | :---: |
| Buses | $5.6-8.0$ |
| Light trucks | $2.0-3.5$ |
| Medium trucks | $6.5-9.3$ |
| Heavy trucks | $6.3-8.8$ |
| Articulated trucks | $6.0-8.5$ |

(Source: World Bank Technical Paper No. 234)
The model uses fixed tear-wear coefficient for the rubber loss volume prediction, (dm3/1000 tire-km)

Unit Costs: Unit costs can either be financial or economic. Various unit costs required are such as:

```
 New Vehicle Price
\(>\) Fuel Cost
\(>\) Lubricants Cost
\(>\) New Tire Cost
\(>\) Crew Time Cost
> Passenger Delay Cost
> Maintenance Labor Cost
```

> Cargo Delay Cost
$>$ Annual Interest Rate
> Overhead per vehicle-km
After giving all required data as input in the software we get following results:

Passenger Car Sample Data (Result Report):- For 100 kmph

| 1. Vehicle Speed | $\mathrm{km} / \mathrm{hr}$ |  | 100 |
| :---: | :---: | :---: | :---: |
| 2. Physical Quantities per 1000 vehicle-km |  |  |  |
| Fuel consumption | liters |  | 85.23 |
| Lubricants <br> consumption | liters | 01.85 |  |
| Tyre wear | \# of equivalent <br> new tires | 00.06 |  |
| Crew time | hours |  | 10.00 |
| Passenger time | hours | 10.00 |  |
| Cargo holding | hours |  | 10.00 |
| Maintenance labor | hours | 02.27 |  |
| Maintenance parts | $\%$ of new vehicle |  |  |
| price | 00.16 |  |  |
| Depreciation | $\%$ of new vehicle |  |  |
| price | 00.43 |  |  |
| Interest | $\%$ of new vehicle |  |  |
| price | 00.23 |  |  |
| 3. Total VOC per <br> 1000 vehicle-km | $\$$ | $\mathbf{1 9 0 . 4 7}$ | $\mathbf{1 0 0 \%}$ |
| Fuel | $\$$ | 98.01 | $51.46 \%$ |
| Lubricants | $\$$ | 12.81 | $06.73 \%$ |
| Tyres | $\$$ | 02.99 | $01.57 \%$ |
| Crew time | $\$$ | 15.00 | $07.88 \%$ |
| Passenger time | $\$$ | 00.00 | $00.00 \%$ |
| Cargo holding | $\$$ | 00.00 | $00.00 \%$ |
| Maintenance labor | $\$$ | 04.99 | $02.62 \%$ |
| Maintenance parts | $\$$ | 10.98 | $05.76 \%$ |
| Depreciation | $\$$ | 29.96 | $15.73 \%$ |
| Interest | $\$$ | 15.73 | $08.26 \%$ |
| Overhead | $\$$ | 00.00 | $00.00 \%$ |

Passenger Car Sample Data (Result Report):- For 120 kmph

| 1. Vehicle Speed | $\mathrm{km} / \mathrm{hr}$ | 120 |
| :---: | :---: | :---: |
| 2. Physical Quantities per 1000 vehicle-km |  |  |
| Fuel consumption | liters | 96.82 |
| Lubricants <br> consumption | liters | 01.85 |
| Tyre wear | \# of equivalent <br> new tires | 00.06 |
| Crew time | hours | 08.33 |


| Passenger time |  | hours | 08.33 |
| :---: | :---: | :---: | :---: |
| Cargo holding |  | hours | 08.33 |
| Maintenance labor |  | hours | 02.27 |
| Maintenance parts |  | new vehicle price | 00.16 |
| Depreciation |  | new vehicle price | 00.41 |
| Interest |  | new vehicle price | 00.21 |
| 3. Total VOC per 1000 vehicle-km | \$ | 198.34 | 100\% |
| Fuel | \$ | 111.34 | 56.14\% |
| Lubricants | \$ | 12.81 | 06.46\% |
| Tyres | \$ | 02.99 | 01.51\% |
| Crew time | \$ | 12.50 | 06.30\% |
| Passenger time | \$ | 00.00 | 00.00\% |
| Cargo holding | \$ | 00.00 | 00.00\% |
| Maintenance labor | \$ | 04.99 | 02.51\% |
| Maintenance parts | \$ | 10.98 | 05.53\% |
| Depreciation | \$ | 28.40 | 14.32\% |
| Interest | \$ | 14.32 | 07.22\% |
| Overhead | \$ | 0.00 | 00.00\% |

## DISCUSSION OF VOC

As clearly shown by tables we can easily observe that the VOC calculated at 100 kmph vehicle speed is lesser than the VOC calculated at vehicle speed 120 kmph . So it is obvious that the design vehicle speed with respect to VOC should be considered as 100 kmph .

## VEHICLES OPERATING COST OF ROB BY IRC

The distance related and time related VOC calculated as following:

## (A)For Speed 100 kmph:

## Distance related economic costs:

## 1. Free Speed (V):

Free Speed (V) = 73.14-(0.711*RF) -0.00171* (RG-2000) Where,
$\mathrm{RG}=2000 \mathrm{~mm} / \mathrm{km}$
$\mathrm{RF}=22.55 \mathrm{~m} / \mathrm{km}$
$\mathrm{V}=73.14-\left(0.711^{*} 22.55\right)-0.00171^{*}(2000-2000)$
$\mathrm{V}=57.11 \mathrm{kmph}$
So, take V = 58 kmph

## 2. Fuel:

Fuel consumption (FC) $=21.85+(504.15 / \mathrm{V})+0.004957^{*} \mathrm{~V}^{2}$ $+0.000652^{* R G}+1.0684 *$ RS-0.3684*FL
Where
$\mathrm{V}=58 \mathrm{~km} / \mathrm{h}$
$\mathrm{RS}=6.77 \mathrm{~m} / \mathrm{km}$
$\mathrm{RG}=2000 \mathrm{~mm} / \mathrm{km}$
$\mathrm{FL}=3.57 \mathrm{~m} / \mathrm{km}$
$(F C)=21.85+(504.15 / 58)+0.004957 * 58^{2}+0.000652^{*} 2000$ + 1.0684*6.77-0.3684*3.57
(FC) = 54.44 liters/ 1000 km
Unit cost of petrol is $76 \mathrm{Rs} /$ liter.
So, the fuel cost $=54.44^{*} 76=\mathbf{4 1 3 7 . 4 4} \mathbf{~ R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$

## 3. Tire cost:

Tire life (TL) = 68771-147.9*RF-26.72*(RG/W)
Where,
$\mathrm{RG}=2000 \mathrm{~mm} / \mathrm{km}$
$\mathrm{W}=7 \mathrm{~m}$
$(\mathrm{TL})=68771-147.9^{*} 22.55-26.72^{*}(2000 / 7)$
$(\mathrm{TL})=57801.57 \mathrm{~km}$
Unit Tire cost $=3250$ Rs
So, the tire cost $=3250 / 57.80157=\mathbf{5 6 . 2 3} \mathbf{R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$

## 4. Lubricants costs:

Engine oil (EOL) $=1.7048+0.03319^{*} \mathrm{RF}+0.0005241^{*}(\mathrm{RG} / \mathrm{W})$
$($ EOL $)=1.7048+0.03319 * 22.55+0.0005241^{*}(2000 / 7)$
(EOL) $=\mathbf{2 . 6}$ liters $/ \mathbf{1 0 0 0} \mathbf{~ k m}$
Other oil (OL) $=1.631+.05167^{* R F}+.001867^{*}(\mathrm{RG} / \mathrm{W})$
(OL) $=1.631+0.05167 * 22.55+.001867 *(2000 / 7)$
$(O L)=3.33$ liters $/ \mathbf{1 0 0 0 0} \mathbf{~ k m}$
Grease (G) $=2.816+0.2007 *$ RF
$(G)=2.816+0.2007 * 22.55$
(G) $=\mathbf{7 . 3 4} \mathbf{~ k g} / \mathbf{1 0 0 0 0} \mathbf{~ k m}$

Unit cost: EOL = $130 \mathrm{Rs} /$ liter

$$
\begin{aligned}
& \mathrm{OL}=248.25 \mathrm{Rs} / \mathrm{liter} \\
& \mathrm{G}=112.35 \mathrm{Rs} / \mathrm{kg}
\end{aligned}
$$

So, the Engine oil cost = 130*2.6 = $\mathbf{3 3 8} \mathbf{~ R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$
Other oil cost $=248.25^{*} .333=\mathbf{8 2 . 6 7} \mathbf{~ R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$ Grease $=112.35^{*} .734=82.46 \mathbf{R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$
So, the Lubricants cost $=338+82.67+82.46=\mathbf{5 0 3 . 1 3}$
Rs/ 1000 km.

## 5. Spare cost

Spare cost (SP) $=\left\{0.0018^{*}(\mathrm{RG}-2000)^{*} 10^{-5}\right\}^{*} \mathrm{NP}$
Where,
$\mathrm{NP}=$ cost of new vehicle $=4,50,600 \mathrm{Rs}$
SP $=\left\{0.0018^{*}(2000-2000)^{*} 10^{-5}\right\}^{*} 450600$
SP = 0.00 paisa $/ \mathrm{km}$

## 6. Maintenance Labor cost

(LC) $=0.5498 *$ SP
Where,
$\mathrm{SP}=0.00$ paisa $/ \mathrm{km}$
So, LC $=0.00$ paisa $/ \mathrm{km}$
So, distance related economic cost $=$
4137.44+56.23+503.13+0.00+0.00 = 4696.8 Rs/1000 km

## Time related economic cost

## 1. Fixed cost

$\mathrm{F}_{\mathrm{x}}=370.14 / \mathrm{UPD}$
Where,
UPD $=6.187^{*} \mathrm{~V}=6.187 * 58=358.846 \mathrm{~km} /$ day
$\mathrm{F}_{\mathrm{x}}=370.14 / 358.846=1.03 \mathrm{Rs} /$ year
So, $F_{x}=1.03 \mathbf{R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$
2. Depreciation cost
$\mathrm{DC}=70.85 / \mathrm{UPD}=70.85 / 358.846=0.19744 \mathrm{Rs} / \mathrm{km}$
So, $D C=197.44$ Rs/1000 km

## 3. Passenger time cost

$\mathrm{PT}=227.82 / \mathrm{V}=227.82 / 58=\mathbf{3 . 9 2 7 9} \mathbf{R s} / \mathbf{k m}$
So, $\mathrm{PT}=3.9279^{*} 1000=3927.9 \mathrm{Rs} / \mathbf{1 0 0 0} \mathbf{~ k m}$

So, time related economic cost $\boldsymbol{= 1 . 0 3 + 1 9 7 . 4 4 + 3 9 2 7 . 9 =}$ 4126.37 Rs/1000 km

Total VOC $=\mathbf{4 6 9 6 . 8} \mathbf{+ 4 1 2 6 . 3 7}=\mathbf{8 8 2 3 . 1 7 5} \mathbf{R s} / \mathbf{1 0 0 0} \mathbf{k m}$
(B) For Speed 120 kmph

## Distance related economic costs

1. Free Speed (V):

Free Speed (V) $=73.14-\left(0.711^{*} R F\right)-0.00171^{*}(R G-2000)$ Where,
$\mathrm{RG}=2000 \mathrm{~mm} / \mathrm{km}$
$\mathrm{RF}=24.97 \mathrm{~m} / \mathrm{km}$
$\mathrm{V}=73.14-\left(0.711^{*} 24.97\right)-0.00171^{*}(2000-2000)$
$\mathrm{V}=55.39 \mathrm{kmph}$
So, take V = 56 kmph

## 2. Fuel

Fuel consumption (FC) $=21.85+(504.15 / \mathrm{V})+0.004957 * \mathrm{~V}^{2}$ $+0.000652 * R G+1.0684 * R S-0.3684 *$ FL
Where,
$\mathrm{V}=56 \mathrm{~km} / \mathrm{h}$
$\mathrm{RS}=10.06 \mathrm{~m} / \mathrm{km}$
$\mathrm{RG}=2000 \mathrm{~mm} / \mathrm{km}$
$\mathrm{FL}=5.55 \mathrm{~m} / \mathrm{km}$
$(F C)=21.85+(504.15 / 56)+0.004957 * 56^{2}$
$+0.000652^{*} 2000+1.0684 * 10.06-0.3684 * 5.55$
(FC) = 56.40 liters $/ \mathbf{1 0 0 0}$ km
Unit cost of petrol is $76 \mathrm{Rs} /$ liter.
So, the fuel cost $=56.40 * 76=\mathbf{4 2 8 6 . 4} \mathbf{R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$
3. Tire cost:

Tire life (TL) $=68771-147.9 *$ RF- $26.72^{*}(R G / W)$
Where,
$\mathrm{RG}=2000 \mathrm{~mm} / \mathrm{km}$
$\mathrm{W}=7 \mathrm{~m}$
$(\mathrm{TL})=68771-147.9^{*} 24.97-26.72^{*}(2000 / 7)$
$(\mathrm{TL})=57443.65 \mathrm{~km}$
Unit Tire cost $=3250$ Rs
So, the tire cost $=3250 / 57.44365=\mathbf{5 6 . 5 8} \mathbf{~ R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$

## 4. Lubricants costs:

Engine oil $(E O L)=1.7048+0.03319^{*} \mathrm{RF}+0.0005241^{*}(\mathrm{RG} / \mathrm{W})$ (EOL) $=1.7048+0.03319 * 24.97+0.0005241 *(2000 / 7)$
(EOL) $=\mathbf{2 . 6 8}$ liters $/ \mathbf{1 0 0 0} \mathbf{~ k m}$
Other oil (OL) $=1.631+.05167^{* R F}+.001867^{*}(\mathrm{RG} / \mathrm{W})$
(OL) $=1.631+0.05167^{*} 24.97+.001867^{*}(2000 / 7)$
$(O L)=\mathbf{3 . 4 5}$ liters $/ \mathbf{1 0 0 0 0} \mathbf{~ k m}$
Grease (G) $=2.816+0.2007 *$ RF
(G) $=2.816+0.2007 * 24.97$
(G) $=7.83 \mathrm{~kg} / 10000 \mathrm{~km}$

Unit cost: EOL = 130 Rs/liter

$$
\begin{aligned}
& \mathrm{OL}=248.25 \mathrm{Rs} / \mathrm{liter} \\
& \mathrm{G}=112.35 \mathrm{Rs} / \mathrm{kg}
\end{aligned}
$$

So, the Engine oil cost = 130*2.68 = 348.4 Rs/1000 km Other oil cost $=248.25^{*} .345=\mathbf{8 5 . 6 4 6} \mathbf{~ R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$ Grease $=112.35^{*} .783=\mathbf{8 7 . 9 7} \mathbf{~ R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$
So, the Lubricants cost $=348.4+85.646+87.97=522.016$ Rs/ $\mathbf{1 0 0 0} \mathbf{~ k m}$

## 5. Spare cost

Spare cost $(S P)=\left\{0.0018^{*}(R G-2000)^{*} 10^{-5}\right\}^{*} N P$
Where,
$\mathrm{NP}=$ cost of new vehicle $=4,50,600 \mathrm{Rs}$
SP $=\left\{0.0018^{*}(2000-2000)^{*} 10^{-5}\right\}^{*} 450600$
$\mathrm{SP}=0.00$ paisa $/ \mathrm{km}$
6. Maintenance Labor cost
(LC) $=0.5498^{*}$ SP
Where,
SP $=0.00$ paisa $/ \mathrm{km}$
So, LC $=0.00$ paisa/km
So, distance related economic cost $=\mathbf{4 2 8 6 . 4 + 5 6 . 5 8}$
$+522.016+0.00+0.00=4864.996$ Rs $/ 1000 \mathrm{~km}$

## Time related economic cost

## 1. Fixed cost

$\mathrm{F}_{\mathrm{x}}=370.14 / \mathrm{UPD}$
Where,
UPD $=6.187^{*} \mathrm{~V}=6.187^{*} 56=346.472 \mathrm{~km} /$ day
$\mathrm{F}_{\mathrm{x}}=370.14 / 346.472=1.068 \mathrm{Rs} /$ year
So, $\mathrm{F}_{\mathrm{x}}=1.068 \mathbf{R s} / \mathbf{1 0 0 0} \mathbf{~ k m}$

## 2. Depreciation cost

DC $=70.85 / \mathrm{UPD}=70.85 / 346.472=0.2045 \mathrm{Rs} / \mathrm{km}$
So, DC = 204.5 Rs/1000 km

## 3. Passenger time cost

$\mathrm{PT}=227.82 / \mathrm{V}=227.82 / 56=4.068 \mathrm{Rs} / \mathbf{k m}$
So, $\mathrm{PT}=4.068^{*} 1000=4068 \mathrm{Rs} / \mathbf{1 0 0 0} \mathbf{~ k m}$
So, time related economic cost $=1.068+204.5+4068=$ 4273.568 Rs/1000 km

Total VOC $=\mathbf{4 8 6 4 . 9 9 6}+4273.568=9138.564 \mathrm{Rs} / \mathbf{1 0 0 0}$ km

Comparison of VOC by IRC and HDM-VOC Model with Their Respective Parameters at 100 kmph

| S.No. | Element | Cost by <br> IRC <br> (Rs/1000 <br> km) | Cost by <br> HDM-V0C <br> $\mathbf{( \$ / 1 0 0 0}$ <br> km) | Cost by <br> HDM-Voc <br> in Rs <br> $\mathbf{( 1 \$ ~ = ~ 6 5 ~}$ <br> Rs) |
| :---: | :---: | :---: | :---: | :---: |
| 1 | Fuel | 4137.445 | 98.01 | 6370.65 |
| 2 | Tire costs | 56.23 | 02.99 | 194.35 |
| 3 | Lubricant cost | 503.13 | 12.81 | 832.65 |


| 4 | Spare cost | 0 | - | - |
| :---: | :---: | :---: | :---: | :---: |
| 5 | Maintenance <br> labor cost | 0 | 4.99 | 324.35 |
| 6 | Fixed cost | 1.03 | - | - |
| 7 | Depreciation <br> cost | 197.44 | 29.96 | 1947.4 |
| 8 | Crew time | - | 15.00 | 975 |
| 9 | Maintenance <br> parts | - | 10.98 | 713.7 |
| 10 | Interest | - | 15.73 | 1022.45 |
| 11 | Overhead | - | 0.00 | 0.00 |
| 12 | Passenger time | 3927.9 | 0.00 | 0.00 |
| 13 | Cargo holding | - | 0.00 | 0.00 |
| 14 | TOTAL VOC | $\mathbf{8 8 2 3 . 1 7 5}$ | $\mathbf{1 9 0 . 4 7}$ | $\mathbf{1 2 3 8 0 . 5 5}$ |

Comparison of VOC by IRC and HDM-VOC Model with Their Respective Parameters at 120 kmph

| S.No. | Element | Cost by <br> IRC <br> (Rs/1000 <br> $\mathbf{k m})$ | Cost by <br> HDM-VOC <br> $\mathbf{( \$ / 1 0 0 0}$ <br> $\mathbf{k m})$ | Cost by <br> HDM-Voc <br> in Rs <br> $\mathbf{( 1 \$ = 6 5}$ <br> Rs) |
| :---: | :---: | :---: | :---: | :---: |
| 1 | Fuel | 4286.4 | 111.34 | 7237.1 |
| 2 | Tire costs | 56.58 | 02.99 | 194.35 |
| 3 | Lubricant cost | 522.016 | 12.81 | 832.65 |
| 4 | Spare cost | 0 | - | - |
| 5 | Maintenance <br> labor cost | 0 | 4.99 | 324.35 |
| 6 | Fixed cost | 1.068 | - | - |
| 7 | Depreciation <br> cost | 204.5 | 28.40 | 1846 |
| 8 | Crew time | - | 12.50 | 812.5 |
| 9 | Maintenance <br> parts | - | 10.98 | 713.7 |
| 10 | Interest | - | 14.32 | 930.8 |
| 11 | Overhead | - | 0.00 | 0.00 |
| 12 | Passenger time | 4068 | 0.00 | 0.00 |
| 13 | Cargo holding | - | 0.00 | 0.00 |
| 14 | TOTAL V0C | $\mathbf{9 1 3 8 . 5 6 4}$ | $\mathbf{1 9 8 . 3 4}$ | $\mathbf{1 2 8 9 1 . 4 5}$ |

## CONCLUSION:

* In this thesis for assessing an arranged ROB permits a snappy and exact survey of vehicle working expense at that ROB. The HDM-VOC model is assessed in light of geometric qualities (levels, length and flat bends and vertical bends), speed and street surface write and condition. The yield of the HDM-VOC display is the VOC while the vehicle goes along a highway at configuration speed. This VOC is framed by various kinds of costs like fuel cost, oil cost, tire cost and so forth.
* Since the utilization of a few parameters in HDMVOC was distinct in compare with IRC rules, so I calculated the VOC according to IRC rules as well. (IRC SP 30:2009)
* By using HDM-VOC demonstrate for ROB at LC-70, Sitapura Jaipur, VOC at configuration speed 100 and 120 kmph has been calculated for a small passenger car. By results we can say that VOC for speed 100 kmph was lesser than what it was for speed 120 kmph . Along these lines the geometric plan of ROB can be improved for the situation configuration speed of 100 kmph .
* The model might be valuable in discovering fuel proficient and naturally well disposed thruways with respect to the fact that the vehicular fuel utilization straight forwardly influences the cost of vehicle task as well as produces ozone harming substances and toxin emissions.
* By the utilization of the IRC rules for ROB at LC-70, Sitapura Jaipur, VOC at speed 100 and 120 kmph is calculated for a small passenger car. By the determination of VOC for these two outline speeds, I can state that VOC for 100 kmph was lesser. Along this the design can further be improved for configuration speed 100 kmph for getting more reduced cost of vehicular operations.


## FUTURE SCOPE:

* VOC can be calculated for all types of vehicles by using this model.
* Fuel demand can easily be determined by using this model.
* VOC calculations always allow us to identify the requirements of highway's repair and maintenance works.
* The modification of further updating of model can also be achieved by these requirements.


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